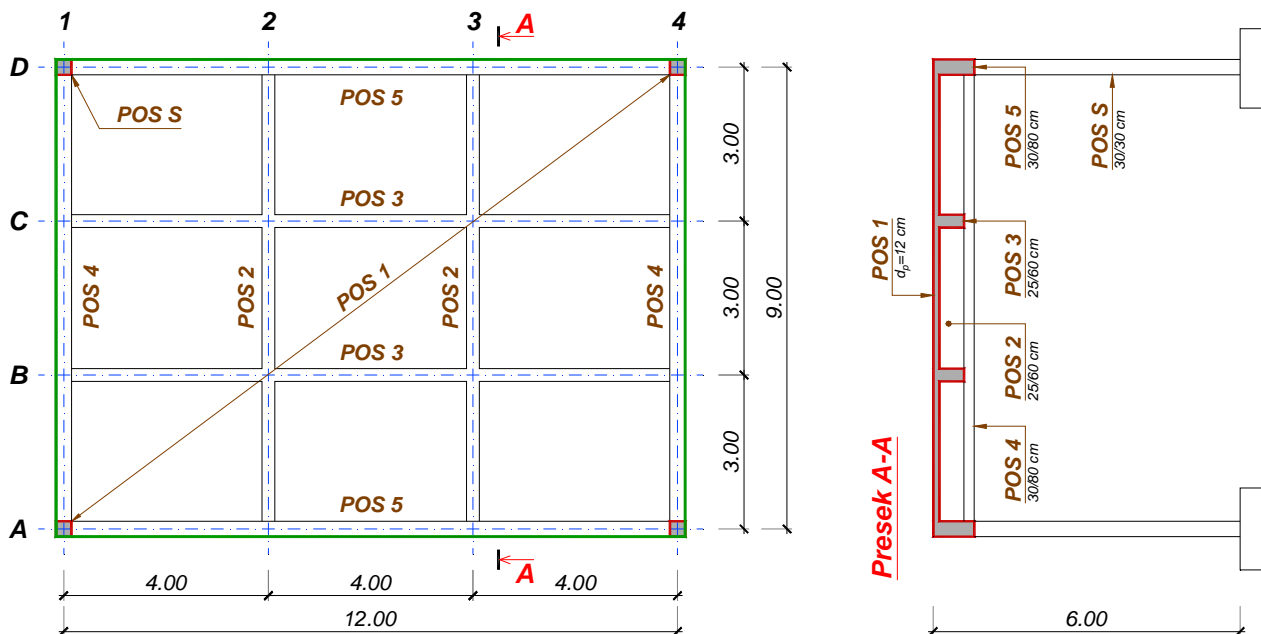


02 Za konstrukciju prikazanu na skici potrebno je:

- 1 Dimenzionisati ploču **POS 1** ($d_p = 12 \text{ cm}$) u karakterističnim presecima.
- 2 Izvršiti analizu opterećenja, sračunati i nacrtati dijagrame statičkih uticaja za grede **POS 2** i **POS 3** ($b/d = 25/60 \text{ cm}$).



- 3 Odrediti potrebnu armaturu greda **POS 2** i **POS 3**. Nacrtati usvojeni raspored armature u poprečnom preseku.
- 4 Izvršiti analizu opterećenja, sračunati i nacrtati dijagrame statičkih uticaja za grede **POS 4** i **POS 5** ($b/d = 30/80 \text{ cm}$). Odrediti potrebnu armaturu greda **POS 4** i **POS 5**.

Povremeno opterećenje $p = 6.0 \text{ kN/m}^2$ deluje po čitavoj površini ploče POS 1. Kvalitet materijala: MB 30, RA 400/500.

1. POS 1 – ploča $d_p = 12 \text{ cm}$

$$g = 0.12 \times 25 = 3.0 \text{ kN/m}^2$$

$$p = 6.0 \text{ kN/m}^2$$

za sve delove ploče:

$$L_y/L_x = 4.0/3.0 = 1.33 \approx 1.3$$

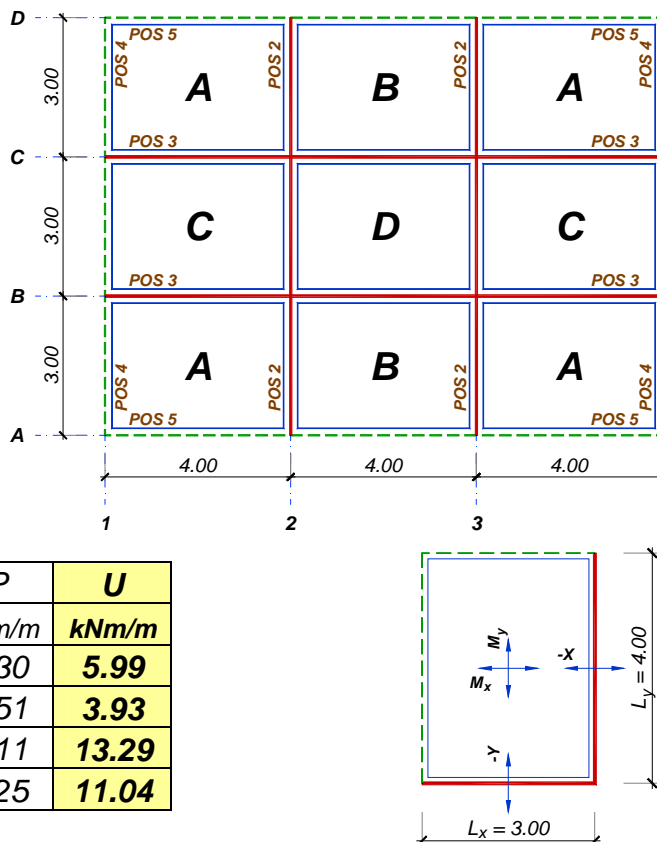
$$G = 3.0 \times 4.0 \times 3.0 = 36 \text{ kN}$$

$$P = 6.0 \times 4.0 \times 3.0 = 72 \text{ kN}$$

$$Q_u = 1.6 \times 36 + 1.8 \times 72 = 187.2 \text{ kN}$$

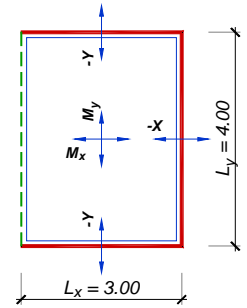
ploča tipa »A«

		G	P	U
k		kNm/m	kNm/m	kNm/m
kraći pravac, polje	0.032 M_x	1.15	2.30	5.99
duži pravac, polje	0.021 M_y	0.76	1.51	3.93
kraći pravac, oslonac	0.071 $-X$	2.56	5.11	13.29
duži pravac, oslonac	0.059 $-Y$	2.12	4.25	11.04



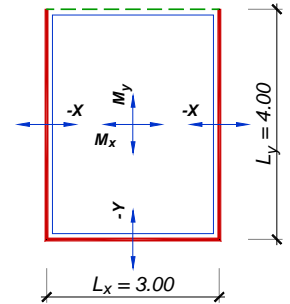
ploča tipa »B«

	<i>k</i>		G kNm/m	P kNm/m	U kNm/m
kraći pravac, polje	0.028	M_x	1.01	2.02	5.24
duži pravac, polje	0.022	M_y	0.79	1.58	4.12
kraći pravac, oslonac	0.063	-X	2.27	4.54	11.79
duži pravac, oslonac	0.055	-Y	1.98	3.96	10.30



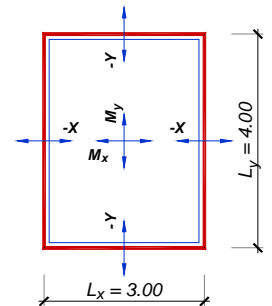
ploča tipa »C«

	<i>k</i>		G kNm/m	P kNm/m	U kNm/m
kraći pravac, polje	0.027	M_x	0.97	1.94	5.05
duži pravac, polje	0.014	M_y	0.50	1.01	2.62
kraći pravac, oslonac	0.057	-X	2.05	4.10	10.67
duži pravac, oslonac	0.044	-Y	1.58	3.17	8.24

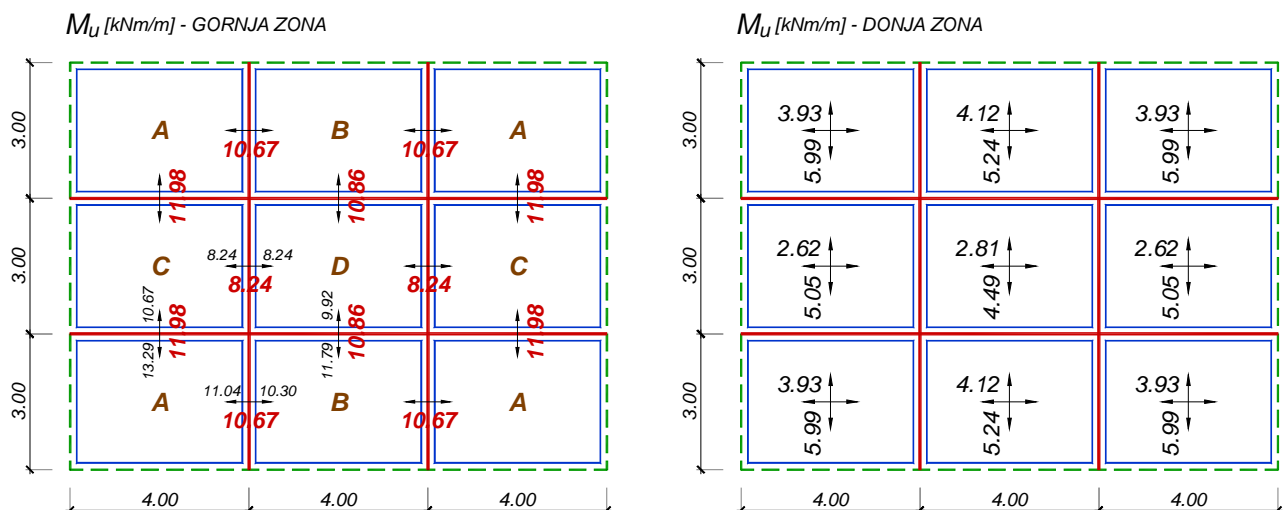


ploča tipa »D«

	<i>k</i>		G kNm/m	P kNm/m	U kNm/m
kraći pravac, polje	0.024	M_x	0.86	1.73	4.49
duži pravac, polje	0.015	M_y	0.54	1.08	2.81
kraći pravac, oslonac	0.053	-X	1.91	3.82	9.92
duži pravac, oslonac	0.044	-Y	1.58	3.17	8.24



Granični momenti savijanja u ploči, posebno za donju, odnosno gornju zonu su prikazani na donjoj šemi. Vrednosti osloničkih momenata susednih ploča su osrednjeni na delovima gde se susstiču dve krstaste ploče različitih konturnih uslova.



S obzirom na veličinu momenata savijanja, prvi red armature donje zone biće postavljen u globalnom Y pravcu ($\max.M_u = 5.99$ kNm/m), preko njega donja zona u X pravcu, zatim gornja zona u X pravcu i konačno gornja zona u Y pravcu ($\max.M_u = 11.98$ kNm/m).

Dimenzionisanje

$$MB\ 30 \Rightarrow f_B = 20.5\ \text{MPa} \quad ; \quad RA\ 400/500 \Rightarrow \sigma_v = 400\ \text{MPa}$$

max. $M_u = M_{yu} = 11.98\ \text{kNm/m}$ (upravno na POS 3 - oslonac između ploča A i C)

$$a_{1y} = 2.5\ \text{cm} \Rightarrow h_y = d - a_{1y} = 12 - 2.5 = 9.5\ \text{cm}$$

$$k = \frac{9.5}{\sqrt{\frac{11.98}{2.05}}} = 3.930 \Rightarrow \varepsilon_b/\varepsilon_a = 1.425/10\text{‰} \quad ; \quad \bar{\mu} = 6.779\%$$

$$A_{ay} = 6.779 \times 9.5 \times \frac{2.05}{40} = 3.30\ \text{cm}^2/\text{m} > A_{a,\min.} = 0.1 \times 12 = 1.2\ \text{cm}^2/\text{m}$$

usvojeno: **RØ8/15** (3.35 cm²/m)

$$A_{ap} = 0.2 \times 3.30 = 0.66\ \text{cm}^2/\text{m} < A_{ap,\min.} = 0.085 \times 12 = 1.02\ \text{cm}^2/\text{m}$$

usvojeno: **RØ8/30** (1.68 cm²/m)

$M_u = 10.86\ \text{kNm/m}$ (upravno na POS 3 - oslonac između ploča B i D)

$$A_{ay} \approx \frac{10.86}{11.98} \times 3.30 = 2.99\ \frac{\text{cm}^2}{\text{m}}$$

usvojeno: **RØ8/15** (3.35 cm²/m)

RØ8/30 (1.68 cm²/m) – podeona armatura

max. $M_{xu} = 10.67\ \text{kNm/m}$ (upravno na POS 2 - oslonac između ploča A i B)

$$h_x = 12 - (2.0 + 0.8 + 0.8/2) = 12 - 3.2 = 8.8\ \text{cm}$$

$$k = \frac{8.8}{\sqrt{\frac{10.67}{2.05}}} = 3.857 \Rightarrow \varepsilon_b/\varepsilon_a = 1.461/10\text{‰} \quad ; \quad \bar{\mu} = 7.043\%$$

$$A_{ax} = 7.043 \times 8.8 \times \frac{2.05}{40} = 3.18\ \text{cm}^2/\text{m}$$

usvojeno: **RØ8/15** (3.35 cm²/m)

RØ8/30 (1.68 cm²/m) – podeona armatura

$M_u = 8.24\ \text{kNm/m}$ (upravno na POS 2 - oslonac između ploča C i D)

$$A_{ay} \approx \frac{8.24}{10.67} \times 3.18 = 2.45\ \frac{\text{cm}^2}{\text{m}}$$

usvojeno: **RØ8/20** (2.51 cm²/m)

RØ8/30 (1.68 cm²/m) – podeona armatura

S obzirom na veoma male vrednosti graničnih računskih momenata savijanja u donjoj zoni, biće usvojena minimalna armatura. Minimalni raspoloživi profil armature je RØ8 a maksimalno dopušteno rastojanje šipki 20 cm, pa sledi:

$$RØ8/20 \Rightarrow A_a = \frac{100 \times a_a^{(1)}}{e_a} = \frac{100 \times 0.503}{20} = 2.51\ \frac{\text{cm}^2}{\text{m}} > A_{a,\min.} = 0.1 \times 12 = 1.2\ \frac{\text{cm}^2}{\text{m}}$$

Nosivost ovako usvojene armature, sračunata sa približnom vrednošću kraka unutrašnjih sila koji odgovara manjoj statičkoj visini $z \approx 0.9 \times 8.8 = 7.92\ \text{cm}$, je:

$$M_u \approx 2.51 \times 7.92 \times 10^{-2} \times 40 = 7.96\ \text{kNm/m} > M_{u,\max} = 5.99\ \text{kNm/m}$$

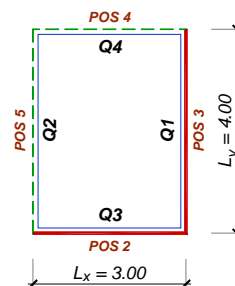
usvojeno: **RØ8/20** (2.51 cm²/m) - donja zona, oba pravca

2. Analiza opterećenja za grede POS 2 – POS 5

Jednako raspodeljeno opterećenje sa pojedinačnih krstastih ploča:

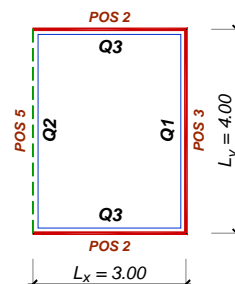
ploča tipa »A«

POS	k	Q	G	P	L	g	p
			kN	kN	m	kN/m	kN/m
3	0.346	Q ₁	12.5	24.9	4.0	3.11	6.23
5	0.233	Q ₂	8.4	16.8	4.0	2.10	4.19
2	0.244	Q ₃	8.8	17.6	3.0	2.93	5.86
4	0.177	Q ₄	6.4	12.7	3.0	2.12	4.25



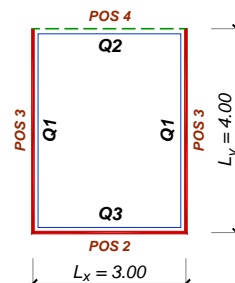
ploča tipa »B«

POS	k	Q	G	P	L	g	p
			kN	kN	m	kN/m	kN/m
3	0.316	Q ₁	11.4	22.8	4.0	2.84	5.69
5	0.218	Q ₂	7.8	15.7	4.0	1.96	3.92
2	0.233	Q ₃	8.4	16.8	3.0	2.80	5.59



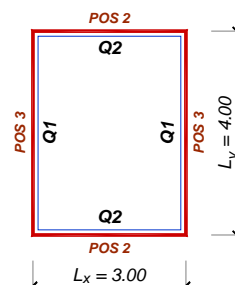
ploča tipa »C«

POS	k	Q	G	P	L	g	p
			kN	kN	m	kN/m	kN/m
3	0.309	Q ₁	11.1	22.2	4.0	2.78	5.56
4	0.165	Q ₂	5.9	11.9	3.0	1.98	3.96
2	0.217	Q ₃	7.8	15.6	3.0	2.60	5.21



ploča tipa »D«

POS	k	Q	G	P	L	g	p
			kN	kN	m	kN/m	kN/m
3	0.291	Q ₁	10.5	21.0	4.0	2.62	5.24
2	0.209	Q ₂	7.5	15.0	3.0	2.51	5.02

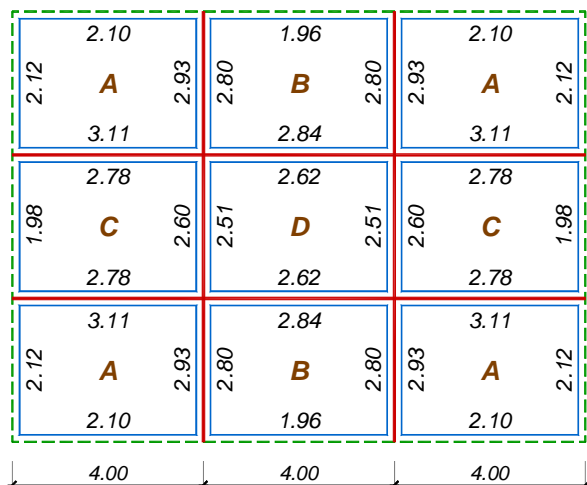


Sopstvenu težinu gređa POS 2 i POS 3, odnosno POS 4 i POS 5:

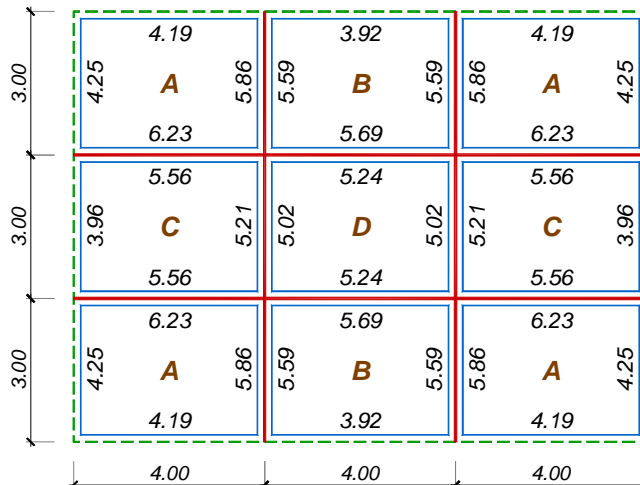
$$g_{2,3} = 0.25 \times 0.6 \times 25 = 3.75 \text{ kN/m} \quad g_{4,5} = 0.3 \times 0.8 \times 25 = 6.0 \text{ kN/m}$$

treba dodati prikazanom stalnom opterećenju sa POS 1.

STALNO OPT. **g** [kN/m]



POVREMENO OPT. **p** [kN/m]



grede POS 2:

$$g_1 = 2.93 + 2.80 + 3.75 = 9.47 \text{ kN/m} = g_3 \quad ; \quad p_1 = 5.86 + 5.59 = 11.45 \text{ kN/m} = p_3$$

$$g_2 = 2.60 + 2.51 + 3.75 = 8.86 \text{ kN/m} \quad ; \quad p_2 = 5.21 + 5.02 = 10.22 \text{ kN/m}$$

grede POS 3:

$$g_1 = 3.11 + 2.78 + 3.75 = 9.65 \text{ kN/m} = g_3 \quad ; \quad p_1 = 6.23 + 5.56 = 11.79 \text{ kN/m} = p_3$$

$$g_2 = 2.84 + 2.62 + 3.75 = 9.21 \text{ kN/m} \quad ; \quad p_2 = 5.69 + 5.24 = 10.93 \text{ kN/m}$$

grede POS 4:

$$g_1 = 2.12 + 6.0 = 8.12 \text{ kN/m} = g_3 \quad ; \quad p_1 = 4.25 \text{ kN/m} = p_3$$

$$g_2 = 1.98 + 6.0 = 7.98 \text{ kN/m} \quad ; \quad p_2 = 3.96 \text{ kN/m}$$

grede POS 5:

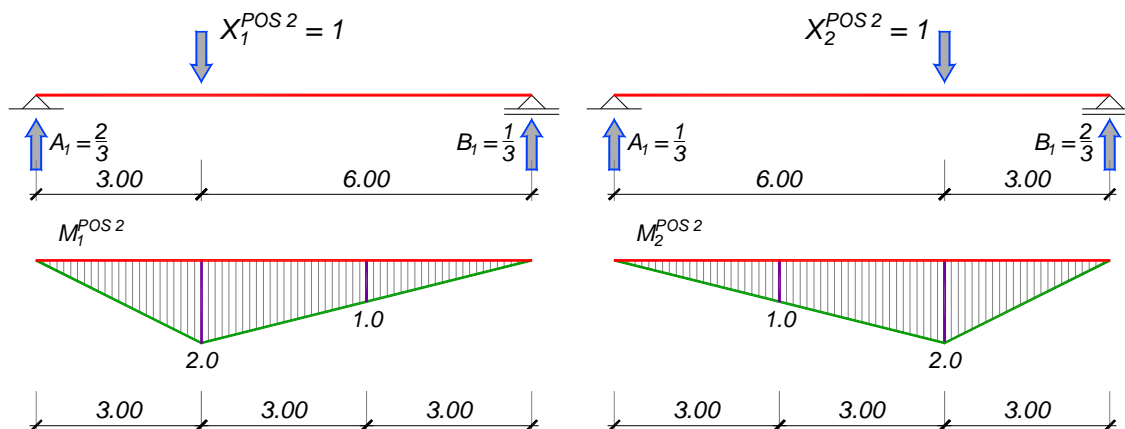
$$g_1 = 2.10 + 6.0 = 8.10 \text{ kN/m} = g_3 \quad ; \quad p_1 = 4.19 \text{ kN/m} = p_3$$

$$g_2 = 1.96 + 6.0 = 7.96 \text{ kN/m} \quad ; \quad p_2 = 3.92 \text{ kN/m}$$

Kod procene statičkog sistema za proračun greda POS 2 do POS 5, treba zapaziti sledeće:

- konstrukcija se oslanja samo na 4 ugaona stuba, pa se mora sračunati kao roštiljna;
- grede POS 2 i POS 3 ($b/d=25/60$ cm) su daleko manje krutosti od greda POS 4 i POS 5 ($b/d=30/80$ cm). Stoga se može smatrati da su grede POS 4 i POS 5 vertikalni oslonci za POS 2 i POS 3;
- torziona krutost greda POS 4 i POS 5 određena je prvenstveno manjom dimenzijom preseka i nije realno da, naročito u stanju sa prslinama, ove grede predstavljaju uklještenja (potpuno spreče rotaciju) krajeva greda POS 2 i POS 3. Slično, ove grede ne mogu jedna drugoj predstavljati uklještenje niti biti uklještenene u stubove datih dimenzija (sa crteža očito $30/30$ cm) pa sledi da su POS 4 i POS 5 proste grede raspona 9.0 odnosno 12.0 m, opterećene opterećenjem sa ploče POS 1 i u trećinama raspona reakcijama greda POS 3, odnosno POS 2;
- grede POS 2 i POS 3 su iste krutosti, opterećene približno jednakim opterećenjem ali različitih raspona, pa se moraju proračunati kao roštiljna konstrukcija. Kao statički nepoznate se usvajaju vertikalne sile u presečnim tačkama, a sračunavaju se metodom sila, izjednačavanjem deformacija presečnih tačaka. Zbog simetrije je jasno da su sve nepoznate sile X_i jednake.

U nastavku je prikazan proračun statičkih uticaja za grede POS 2 i POS 3 usled stalnog (G) i povremenog (P) opterećenja.



$$EI\delta_{11}^{\text{POS}2} = \frac{9.0}{3} \times 2.0^2 = 12.0 = EI\delta_{22}^{\text{POS}2}$$

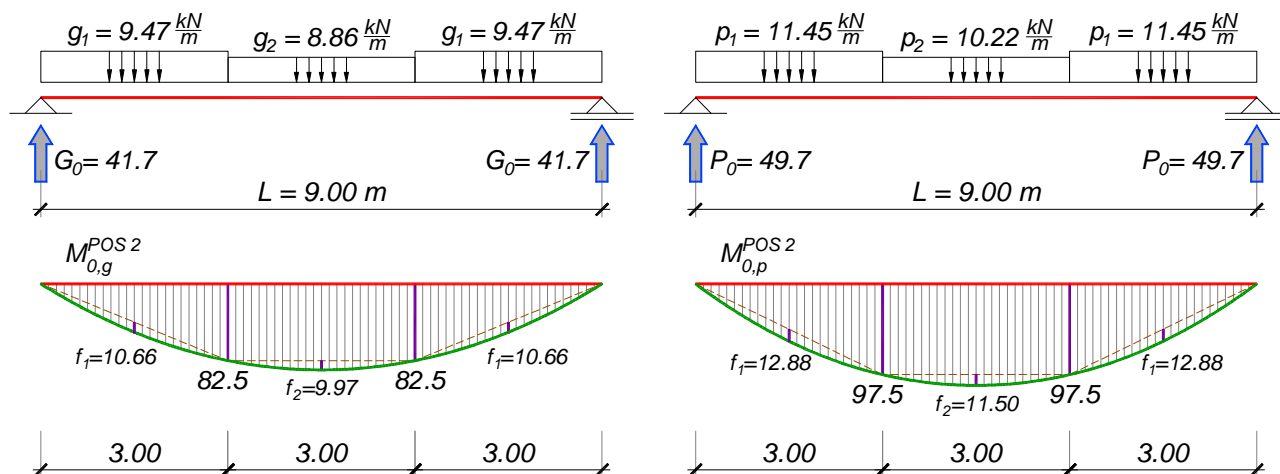
$$EI\delta_{12}^{POS2} = 2 \times \frac{3.0}{3} \times 2.0 \times 1.0 + \frac{3.0}{6} \times [2.0 \times (2 \times 1.0 + 2.0) + 1.0 \times (2 \times 2.0 + 1.0)] = 10.5 = EI\delta_{21}^{POS2}$$

$$G_0 = \frac{2 \times 9.47 + 8.86}{2} \times 3.0 = 41.7 \text{ kN} \quad ; \quad P_0 = \frac{2 \times 11.45 + 10.22}{2} \times 3.0 = 49.7 \text{ kN}$$

$$M_{0g} = 41.7 \times 3.0 - 9.47 \times 3.0^2 / 2 = 82.5 \text{ kNm} \quad ; \quad M_{0p} = 49.7 \times 3.0 - 11.45 \times 3.0^2 / 2 = 97.5 \text{ kNm}$$

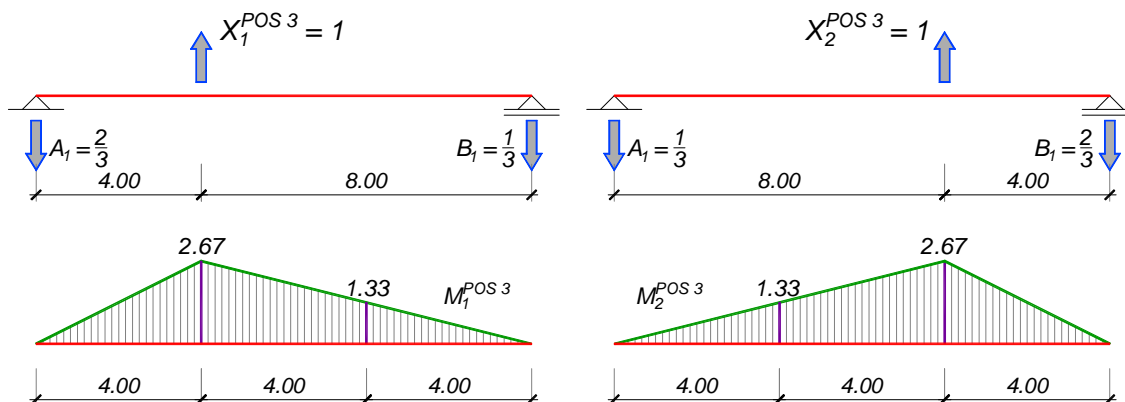
$$f_{1g} = 9.47 \times 3.0^2 / 8 = 10.66 \text{ kNm} \quad ; \quad f_{1p} = 11.45 \times 3.0^2 / 8 = 12.88 \text{ kNm}$$

$$f_{2g} = 8.86 \times 3.0^2 / 8 = 9.97 \text{ kNm} \quad ; \quad f_{2p} = 10.22 \times 3.0^2 / 8 = 11.50 \text{ kNm}$$



$$EI\delta_{10,g}^{POS2} = \frac{3.0}{3} \times (82.5 \times 2.0 + 82.5 \times 1.0) + \frac{3.0}{3} \times 10.66 \times (1.0 + 2.0) + \frac{3.0}{2} \times 82.5 \times (2.0 + 1.0) + \frac{3.0}{3} \times 9.97 \times (2.0 + 1.0) = 680.7 = EI\delta_{20,g}^{POS2}$$

$$EI\delta_{10,p}^{POS2} = \frac{3.0}{3} \times (97.5 \times 2.0 + 97.5 \times 1.0) + \frac{3.0}{3} \times 12.88 \times (1.0 + 2.0) + \frac{3.0}{2} \times 97.5 \times (2.0 + 1.0) + \frac{3.0}{3} \times 11.50 \times (2.0 + 1.0) = 804.6 = EI\delta_{20,p}^{POS2}$$



$$EI\delta_{11}^{POS3} = \frac{12.0}{3} \times 2.67^2 = 28.44 = EI\delta_{22}^{POS3}$$

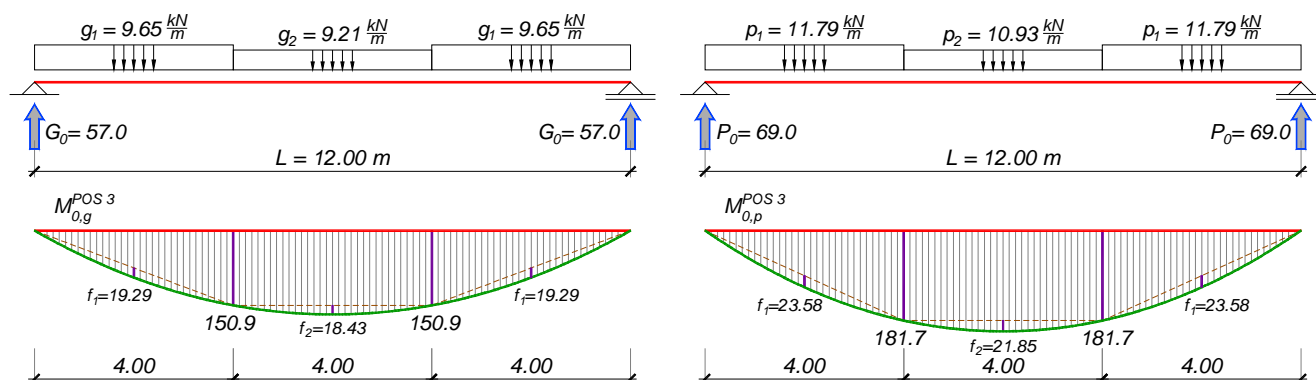
$$EI\delta_{12}^{POS3} = 2 \times \frac{4.0}{3} \times 2.67 \times 1.33 + \frac{4.0}{6} \times [2.67 \times (2 \times 1.33 + 2.67) + 1.33 \times (2 \times 2.67 + 1.33)] = 24.89$$

$$G_0 = \frac{2 \times 9.65 + 9.21}{2} \times 4.0 = 57.0 \text{ kN} \quad ; \quad P_0 = \frac{2 \times 11.79 + 10.93}{2} \times 4.0 = 69.0 \text{ kN}$$

$$M_{0g} = 57 \times 4.0 - 9.65 \times 4.0^2 / 2 = 150.9 \text{ kNm} ; \quad M_{0p} = 69 \times 4.0 - 11.79 \times 4.0^2 / 2 = 181.7 \text{ kNm}$$

$$f_{1g} = 9.65 \times 4.0^2 / 8 = 19.29 \text{ kNm} ; \quad f_{1p} = 11.79 \times 4.0^2 / 8 = 23.58 \text{ kNm}$$

$$f_{2g} = 9.21 \times 4.0^2 / 8 = 18.43 \text{ kNm} ; \quad f_{2p} = 10.93 \times 4.0^2 / 8 = 21.85 \text{ kNm}$$



$$-EI\delta_{10,g}^{POS3} = \frac{4.0}{3} \times (150.9 \times 2.67 + 150.9 \times 1.33) + \frac{4.0}{3} \times 19.29 \times (1.33 + 2.67) +$$

$$+ \frac{4.0}{2} \times 150.9 \times (2.67 + 1.33) + \frac{4.0}{3} \times 18.43 \times (2.67 + 1.33) = 2212.7 = -EI\delta_{20,g}^{POS3}$$

$$-EI\delta_{10,p}^{POS3} = \frac{4.0}{3} \times (181.7 \times 2.67 + 181.7 \times 1.33) + \frac{4.0}{3} \times 23.58 \times (1.33 + 2.67) +$$

$$+ \frac{4.0}{2} \times 181.7 \times (2.67 + 1.33) + \frac{4.0}{3} \times 21.85 \times (2.67 + 1.33) = 2665.3 = -EI\delta_{20,p}^{POS3}$$

$$EI\delta_{11} = EI\delta_{11}^{POS2} + EI\delta_{11}^{POS3} = 12.0 + 28.44 = 40.44 = EI\delta_{22}$$

$$EI\delta_{12} = EI\delta_{12}^{POS2} + EI\delta_{12}^{POS3} = 10.5 + 24.89 = 35.39 = EI\delta_{21}$$

a. stalno opterećenje

$$EI\delta_{10,g} = EI\delta_{10,g}^{POS2} + EI\delta_{10,g}^{POS3} = 680.7 - 2212.7 = -1531.9 = EI\delta_{20,g}$$

$$\left. \begin{aligned} EI\delta_{11} \cdot X_{1g} + EI\delta_{12} \cdot X_{2g} + EI\delta_{10,g} &= 0 \\ EI\delta_{21} \cdot X_{1g} + EI\delta_{22} \cdot X_{2g} + EI\delta_{20,g} &= 0 \end{aligned} \right\} \Rightarrow X_{1g} = -\frac{-1531.9}{40.44 + 35.39} = 20.2 \text{ kN} = X_{2g}$$

Sile koje se prenose na POS 5 (sa POS 2), odnosno POS 4 (sa POS 3):

$$G^{POS2} = 41.7 + \frac{2}{3} \times 20.2 + \frac{1}{3} \times 20.2 = 61.9 \text{ kN}$$

$$G^{POS3} = 57.0 + \left(-\frac{2}{3}\right) \times 20.2 + \left(-\frac{1}{3}\right) \times 20.2 = 36.8 \text{ kN}$$

b. povremeno opterećenje

$$EI\delta_{10,p} = EI\delta_{10,p}^{POS2} + EI\delta_{10,p}^{POS3} = 804.6 - 2665.3 = -1860.8 = EI\delta_{20,p}$$

$$X_{1p} = -\frac{-1860.8}{40.44 + 35.39} = 24.5 \text{ kN} = X_{2p}$$

Sile koje se prenose na POS 5 (sa POS 2), odnosno POS 4 (sa POS 3):

$$P^{POS2} = 49.7 + \frac{2}{3} \times 24.5 + \frac{1}{3} \times 24.5 = 74.2 \text{ kN}$$

$$P^{POS3} = 69.0 + \left(-\frac{2}{3}\right) \times 24.5 + \left(-\frac{1}{3}\right) \times 24.5 = 44.5 \text{ kN}$$

3. Dimenzionisanje POS 2 i POS 3

Dimenzionisanje POS 2

$$q_{u1} = 1.6 \times 9.47 + 1.8 \times 11.45 = 35.76 \text{ kN/m}$$

$$q_{u2} = 1.6 \times 8.86 + 1.8 \times 10.22 = 32.58 \text{ kN/m}$$

$$X_u = 1.6 \times 20.2 + 1.8 \times 24.5 = 76.5 \text{ kN}$$

$$A_u = \frac{2 \times 35.76 + 32.58}{2} \times 3.0 + 76.5 = 232.7 \text{ kN}$$

$$T_u^{B,l} = 232.7 - 35.76 \times 3.0 = 125.4 \text{ kN}$$

$$T_u^{B,d} = 125.4 - 76.5 = 48.9 \text{ kN}$$

$$M_u^B = 232.7 \times 3.0 - \frac{35.76 \times 3.0^2}{2} = 537.0 \text{ kNm}$$

$$M_{u,\max} = 537 + 48.9 \times \frac{3}{2} - \frac{32.58 \times 3^2}{8} = 573.7 \text{ kNm}$$

$$B = \min. \left\{ \begin{array}{l} 25 + 0.25 \times 900 = 250 \\ 25 + 20 \times 12 = 265 \end{array} \right\} = 250 \text{ cm}$$

$$\text{pretp. } a_1 = 7 \text{ cm} \Rightarrow h = 60 - 7 = 53 \text{ cm}$$

$$k = \frac{53}{\sqrt{\frac{573.7 \times 10^2}{250 \times 2.05}}} = 5.009 \Rightarrow \varepsilon_b / \varepsilon_a = 1.051 / 10\text{‰}$$

$$s = 0.095 \Rightarrow x = 0.095 \times 53 = 5.0 \text{ cm} < 12 \text{ cm}$$

$$\bar{\mu} = 4.123\%$$

$$A_a = 4.123 \times \frac{250 \times 53}{100} \times \frac{2.05}{40} = 28.00 \text{ cm}^2 \Rightarrow \text{usvojeno } \mathbf{6R\text{Ø}25} \text{ (29.45 cm}^2\text{)}$$

$$\tau_n^A = \frac{232.7}{25 \times 0.9 \times 53} = 0.195 \frac{\text{kN}}{\text{cm}^2} \left\{ \begin{array}{l} > \tau_r = 1.1 \text{ MPa} \\ < 3\tau_r \end{array} \right.$$

$$\tau_n^{B,l} = \frac{125.4}{25 \times 0.9 \times 53} = 0.105 \frac{\text{kN}}{\text{cm}^2} = 1.05 \text{ MPa} < \tau_r$$

$$\lambda = \frac{1.95 - 1.1}{1.95 - 1.05} \times 3.0 = 2.838 \text{ m}$$

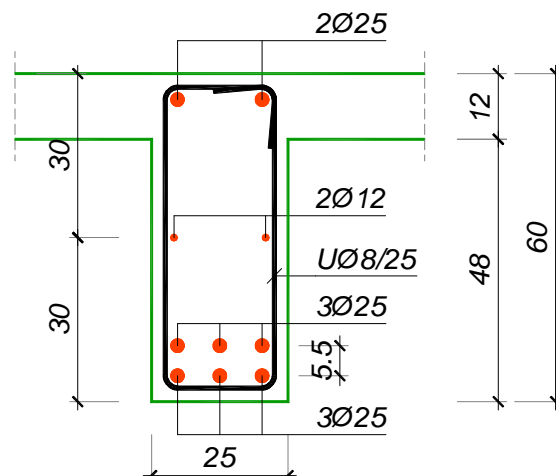
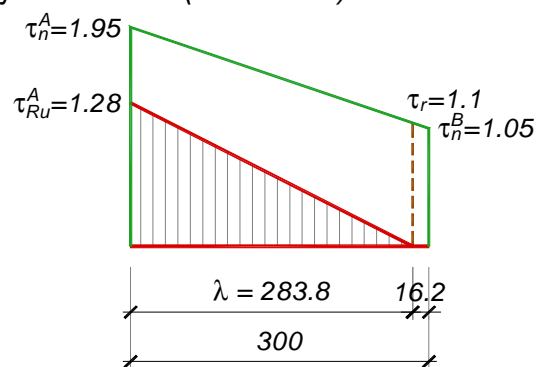
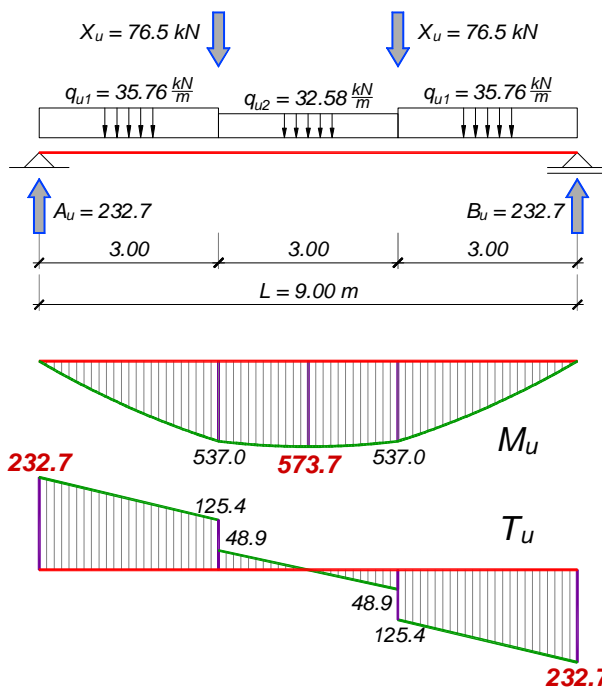
$$\tau_{Ru}^A = \frac{3}{2} \times (0.195 - 0.11) = 0.128 \frac{\text{kN}}{\text{cm}^2}$$

$$e_u = \frac{2 \times 0.503}{25 \times 0.128} \times 40 = 12.6 \text{ cm}$$

usvojeno **URØ8/12.5** (m=2)

$$\Delta A_a = \frac{232.7}{2 \times 40} \times (\cot 45^\circ - \cot 90^\circ) = 2.91 \text{ cm}^2$$

Preko slobodnog oslonca prevodi se minimalno trećina armature iz polja, tj. **2RØ25**.



Dimenzionisanje POS 3

$$q_{u1} = 1.6 \times 9.65 + 1.8 \times 11.79 = 36.65 \text{ kN/m}$$

$$q_{u2} = 1.6 \times 9.21 + 1.8 \times 10.93 = 34.41 \text{ kN/m}$$

$$X_u = 1.6 \times 20.2 + 1.8 \times 24.5 = 76.5 \text{ kN}$$

$$A_u = \frac{2 \times 36.65 + 34.41}{2} \times 4 - 76.5 = 138.9 \text{ kN}$$

$$T_u^{B,l} = 138.9 - 36.65 \times 4.0 = -7.7 \text{ kN}$$

$$T_u^{B,d} = -7.7 - (-76.5) = 68.8 \text{ kN}$$

$$M_u^B = 138.9 \times 4 - \frac{36.65 \times 4^2}{2} = 262.5 \text{ kNm}$$

$$M_{u,max} = 262.5 + 68.8 \times \frac{4}{2} - \frac{34.41 \times 4^2}{8} = 331.3 \text{ kNm}$$

$$B = \min. \left\{ \begin{array}{l} 25 + 0.25 \times 1200 = 325 \\ 25 + 20 \times 12 = 265 \end{array} \right\} = 265 \text{ cm} ; \text{ pretp. } a_1 = 6 \text{ cm} \Rightarrow h = 60 - 6 = 54 \text{ cm}$$

$$k = \frac{54}{\sqrt{\frac{331.3 \times 10^2}{265 \times 2.05}}} = 6.914 \Rightarrow s = 0.067 \Rightarrow x = 0.067 \times 54 = 3.6 \text{ cm} < 12 \text{ cm}$$

$$\varepsilon_b / \varepsilon_a = 0.723 / 10\%$$

$$\bar{\mu} = 2.141\%$$

$$A_a = 2.141 \times \frac{265 \times 54}{100} \times \frac{2.05}{40} = 15.71 \text{ cm}^2 \Rightarrow \text{usvojeno } 5R\emptyset 22 \text{ (19.01 cm}^2\text{)}$$

$$\tau_n^A = \frac{138.9}{25 \times 0.9 \times 54} = 0.114 \frac{\text{kN}}{\text{cm}^2} > \tau_r$$

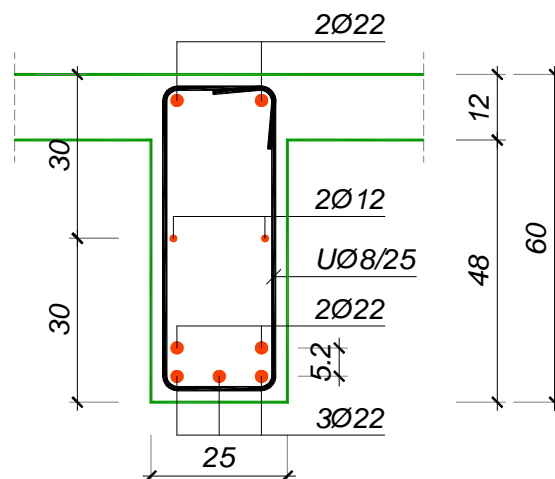
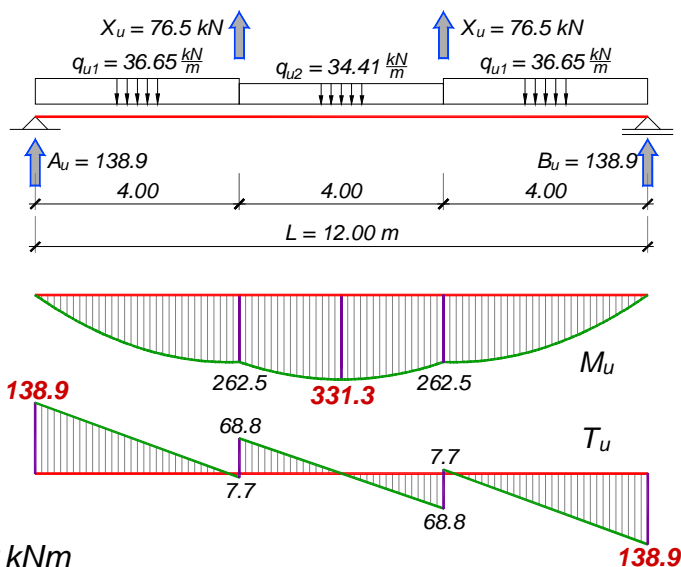
$$\tau_{Ru}^A = \frac{3}{2} \times (0.114 - 0.11) = 0.007 \frac{\text{kN}}{\text{cm}^2}$$

$$e_u \leq \frac{m \times a_u^{(1)}}{b \times \mu_{uz,min}} = \frac{2 \times 0.503}{25 \times 0.2\%} = 20.1 \text{ cm}$$

usvojeno UR \emptyset 8/20 (m=2)

$$\Delta A_a = \frac{138.9}{2 \times 40} \times (\cot 45^\circ - \cot 90^\circ) = 1.74 \text{ cm}^2$$

usvojeno 2R \emptyset 22 (7.60 cm²)

**4. Proračun statičkih uticaja i dimenzionisanje POS 4 i POS 5**Proračun POS 4

$$q_{u1} = 1.6 \times 8.12 + 1.8 \times 4.25 = 20.64 \text{ kN/m} ; \quad q_{u2} = 1.6 \times 7.98 + 1.8 \times 3.96 = 19.90 \text{ kN/m}$$

$$Q_u = 1.6 \times 36.8 + 1.8 \times 44.5 = 138.9 \text{ kN}$$

$$A_u = \frac{2 \times 20.64 + 19.90}{2} \times 3.0 + 138.9 = 230.7 \text{ kN}$$

$$T_u^{B,l} = 230.7 - 20.64 \times 3.0 = 168.8 \text{ kN} ; \quad T_u^{B,d} = 168.8 - 138.9 = 29.8 \text{ kN}$$

$$M_u^B = 230.7 \times 3.0 - \frac{20.64 \times 3.0^2}{2} = 599.3 \text{ kNm}$$

$$M_{u,\max} = 599.3 + 29.8 \times \frac{3}{2} - \frac{19.90 \times 3^2}{8} = 621.6 \text{ kNm}$$

$$B = \min. \left\{ \begin{array}{l} 30 + 0.25 \times 900 / 3 = 105 \\ 30 + 8 \times 12 = 126 \end{array} \right\} = 105 \text{ cm}$$

$$\text{pretp. } a_1 = 7 \text{ cm} \Rightarrow h = 80 - 7 = 73 \text{ cm}$$

$$k = \frac{73}{\sqrt{\frac{621.6 \times 10^2}{105 \times 2.05}}} = 4.296$$

$$\varepsilon_b / \varepsilon_a = 1.271 / 10\text{‰}$$

$$s = 0.113 \Rightarrow x = 0.113 \times 73 = 8.2 \text{ cm} < 12 \text{ cm}$$

$$\bar{\mu} = 5.646\%$$

$$A_a = 5.646 \times \frac{105 \times 73}{100} \times \frac{2.05}{40} = 22.18 \text{ cm}^2 \Rightarrow \text{usvojeno } \mathbf{6R\text{\O}22} (22.81 \text{ cm}^2)$$

$$\tau_n^A = \frac{230.7}{30 \times 0.9 \times 73} = 0.117 \frac{\text{kN}}{\text{cm}^2} > \tau_r$$

$$\tau_{Ru}^A = \frac{3}{2} \times (0.117 - 0.11) = 0.011 \frac{\text{kN}}{\text{cm}^2}$$

$$e_u \leq \frac{m \times a_u^{(1)}}{b \times \mu_{uz,\min}} = \frac{2 \times 0.503}{30 \times 0.2\%} = 16.8 \text{ cm}$$

usvojeno $\mathbf{UR\text{\O}8/15}$ ($m=2$)

$$\Delta A_a = \frac{230.7}{2 \times 40} \times (\cot 45^\circ - \cot 90^\circ) = 2.88 \text{ cm}^2$$

usvojeno $\mathbf{2R\text{\O}22}$ (7.60 cm^2)

Proračun POS 5

$$q_{u1} = 1.6 \times 8.10 + 1.8 \times 4.19 = 20.50 \text{ kN/m} \quad ; \quad q_{u2} = 1.6 \times 7.96 + 1.8 \times 3.92 = 19.80 \text{ kN/m}$$

$$Q_u = 1.6 \times 61.9 + 1.8 \times 74.2 = 232.7 \text{ kN}$$

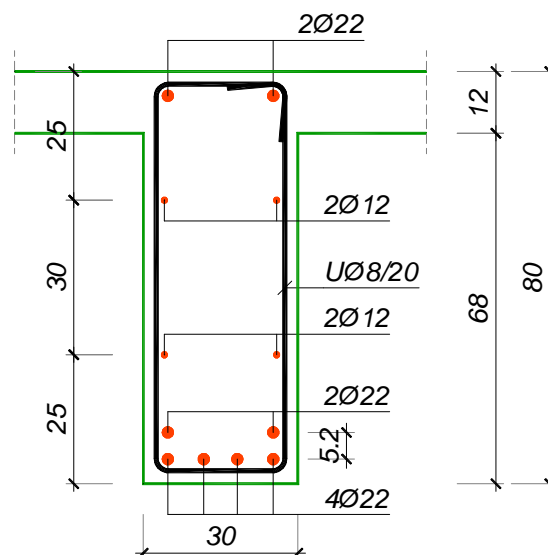
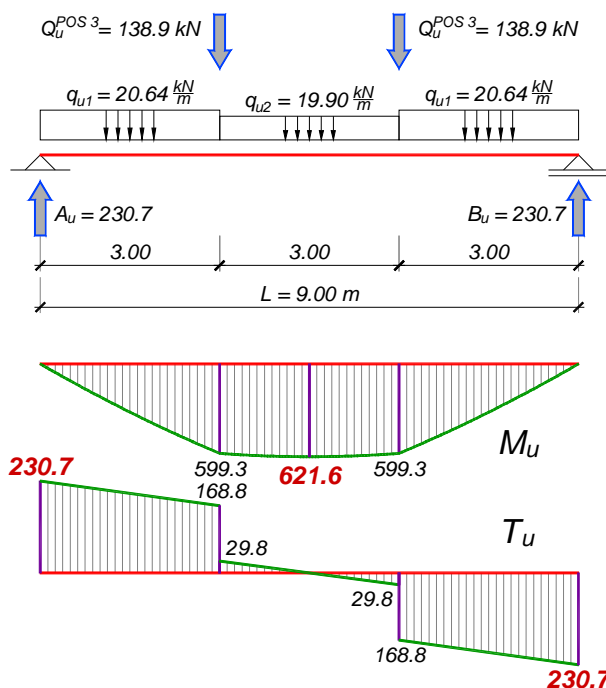
$$A_u = \frac{2 \times 20.50 + 19.80}{2} \times 4.0 + 232.7 = 354.3 \text{ kN}$$

$$T_u^{B,l} = 354.3 - 20.50 \times 4.0 = 272.3 \text{ kN} \quad ; \quad T_u^{B,d} = 272.3 - 232.7 = 39.6 \text{ kN}$$

$$M_u^B = 354.3 \times 4.0 - \frac{20.50 \times 4.0^2}{2} = 1253.1 \text{ kNm}$$

$$M_{u,\max} = 1253.1 + 39.6 \times \frac{4.0}{2} - \frac{19.80 \times 4.0^2}{8} = 1292.7 \text{ kNm}$$

$$B = \min. \left\{ \begin{array}{l} 30 + 0.25 \times 1200 / 3 = 130 \\ 30 + 8 \times 12 = 126 \end{array} \right\} = 126 \text{ cm}$$



pretp. $a_1 = 9 \text{ cm} \Rightarrow h = 80 - 9 = 71 \text{ cm}$

$$k = \frac{71}{\sqrt{\frac{1292.7 \times 10^2}{126 \times 2.05}}} = 3.174$$

$$\varepsilon_b / \varepsilon_a = 1.927 / 10\text{‰}$$

$$s = 0.162 \Rightarrow x = 0.162 \times 71 = 11.5 \text{ cm} < 12 \text{ cm}$$

$$\bar{\mu} = 10.564\%$$

$$A_a = 10.564 \times \frac{126 \times 71}{100} \times \frac{2.05}{40} = 48.43 \text{ cm}^2$$

usvojeno **10RØ25** (49.09 cm²)

$$\tau_n^A = \frac{354.3}{30 \times 0.9 \times 73} = 0.185 \frac{\text{kN}}{\text{cm}^2} \left\{ \begin{array}{l} > \tau_r \\ < 3\tau_r \end{array} \right.$$

$$\tau_n^{B,I} = \frac{272.3}{30 \times 0.9 \times 73} = 0.142 \frac{\text{kN}}{\text{cm}^2} > \tau_r$$

Dužina osiguranja $\lambda = 4.0 \text{ m}$.

$$\tau_{Ru}^A = \frac{3}{2} \times (0.185 - 0.11) = 0.112 \frac{\text{kN}}{\text{cm}^2}$$

$$e_u = \frac{2 \times 0.503}{30 \times 0.112} \times 40 = 11.9 \text{ cm}$$

usvojeno **URØ8/10** (m=2)

$$\Delta A_a = \frac{354.3}{2 \times 40} \times (\cot 45^\circ - \cot 90^\circ) = 4.43 \text{ cm}^2$$

usvojeno **4RØ25** (19.63 cm²)

